Introduction

Landslides are often assumed to exhibit self-similarity in their failure geometry and a linear scaling between slip depth and rupture length. Such an assumption has important implications for the prediction of large landslide volumes and for the estimation of erosion budgets by mass-wasting. However, some field data indicate a break from selfsimilarity and imply that in some circumstances landslide depth may scale non-linearly with length. Here we test the simple scaling hypothesis by numerical experiment.

Previous Studies

The self-similar length-depth scaling hypothesis was first made in the context of landslide area-volume distributions and sediment budget estimation by Hovius et al. (1997) and has been used on several occasions since (e.g., Guzzetti et al., 2009; Lavé and Burbank, 2004; Malamud et al., 2004b). It is often implicit in engineering assessments of slope failure, particularly in modeling treatments that impose an anticipated slip plane in the assessment of a safety factor (e.g., Morgenstern and Price, 1965; Wyllie and Mah, 2004). Linear scaling is also broadly consistent with geotechnical modeling where the plane of failure it not predetermined, for example in studies using commercial codes such as FLAC, FLAC-3D, ELFEN, PFC, Tochnog and UDEC (Barla, 2008; Brideau et al., 2006; Commend et al., 2004; Crosta et al., 2003; Crosta and Clague, 2006; Evans et al., 2006; Li et al., 2006; Pasculli and Sciarra, 2006; Poisel et al., 2009; Stead et al., 2006).

Model Setup

- Used SNAC (StGermaiN Analysis of Continua), a 3D community code originally designed to model elastoviscoplastic deformation on a crustal scale (Choi et al., 2008).
- Landslide rupture treated as emergent *shear localization* under strain-weakening Mohr-Coulomb plasticity.
- Only the initial slip and early motion of a landslide simulated.
- 2D simulations of failures spanning 50, 100, 200, and 400 m.
- Density: 2500 kg/m³; Lamé's constants: 30 GPa; friction angle: 25° ; dilation angle: 5° ; cohesion: 10 to 30 kPa, reduced to zero over plastic strain of 100 %.

References Barla, G. B., February 2008. Editorial - numerical and physical modelling of massive rock slope failure. Rock Mechanics and Rock Engineering 41 (1), 1-2, doi:10.1007/s00603-008-0164-0.

Brideau, M.-A., Stead, D., Couture, R., 2006. Structural and engineering geology of the East Gate Landslide, Purcell Mountains, British Columbia, Canada. Engineering Geology 84, 183–206, doi:10.1016/j.enggeo.2006.01.004.

Choi, E., Lavier, L., Gurnis, M., 2008. Thermomechanics of mid-ocean ridge segmentation. Physics of the Earth and Planetary Interiors 171, 374–386, doi:10.1016/j.pepi.2008.08.010.

Commend, S., Geiser, F., Tacher, L., 2004. 3D numerical modeling of a landslide in Switzerland. In: Pande, G. N., Pietruszczak, S. (Eds.), Numerical Models in Geomechanics: Proceedings of the Ninth International Sym-

Landslide Rupture and Length-Depth Scaling

Colin Stark and Eunseo Choi^{*} (*email: echoi@ldeo.columbia.edu) Lamont-Doherty Earth Observatory, P.O. Box 1000, 61 Rt. 9W, Palisades, NY 10964, USA

Hypothesis

In the absence of material and topographic complexity, the maximum depth of landslide rupture scales linearly with rupture length:

$$z_{\rm m} = \eta L \tag{1}$$

where $z_{\rm m}$ is the maximum rupture depth (measured vertically), L is the planform rupture length (measured horizontally). This is derived from the following simple scaling of mobilized debris volume V and rupture area A: $V \sim A^{3/2}$ and $V \sim Az_m$. Since $A \sim L^2$,

$$z_{\rm m} \sim A^{\frac{1}{2}} \sim L$$
.



posium on 'Numerical Models in Geomechanics - Numog IX', Ottawa, Canada, 25-27 August 2004. Taylor and Francis, London, pp. 595-601. Crosta, G. B., Imposimato, S., Roddeman, D. G., 2003. Numerical modelling of large landslides stability and runout. Natural Hazards and Earth System Sciences 3, 523-538.

Crosta, G. B., Clague, J. J., 2006. Large landslides: Dating, triggering, modelling, and hazard assessment. Engineering Geology 83, 1-3. Evans, S. G., Mugnozza, G. S., Strom, A., Herrmanns, R. L. (Eds.), 2006. Landslides from massive rock slope failure. NATO Science Series. Springer.

Guzzetti, F., Ardizzone, F., Cardinali, M., Rossi, M., Valigi, D., 2009. Landslide volumes and landslide mobilization rates in Umbria, central Italy. Earth and Planetary Science Letters 279, 222-229, doi:10.1016/j.epsl.2009.01.005.







Summary

- A linear scaling confirmed over the tested values of L.
- A depth-length ratio of 11-15% is recorded.
- Interesting complexity in the evolution of the slip • 3D continuum modeling of soil and rock-slope failplane found: Failure initiates at the toe, propure, and the study of their rich behavior, is now agates upslope, and asymptotically parallels the feasible using a non-commercial code on superplanar upper boundary. computing platforms.

Hovius, N., Stark, C. P., Allen, P. A., 1997. Sediment flux from a mountain belt derived by landslide mapping. Geology 25 (3), 231–234. Lavé, J., Burbank, D., 2004. Denudation processes and rates in the Transverse Ranges, southern California: Erosional response of a transitional landscape to external and anthropogenic forcing. Journal of Geophysical Research 109, F01006, doi:10.1029/2003JF000023.

Li, X.-Z., Kong, J.-M., Xu, Q., 2006. Deformation and development tendency of Shiliushubao landslide by numerical modeling. Wuhan University Journal of Natural Sciences 11 (4), 840-846.

Malamud, B. D., Turcotte, D. L., Guzzetti, F., Reichenbach, P., 2004b. Landslides, earthquakes, and erosion. Earth and Planetary Science Letters 229, 45–59, doi:10.1016/j.epsl.2004.10.018. Morgenstern, N. R., Price, V. E., 1965. The analysis of the stability of gen-

eral slip surfaces. Géotechnique 15 (1), 79-93.

mechanical parameters compared with the results of a probabilistic approach assuming selected heterogeneities at different spatial scales. Giornale di Geologia Applicata 3, 269–280, doi:10.1474/GGA.2006-03.3-35.0128.Poisel, R., Angerer, H., Pöllinger, M., Kalcher, T., Kittl, H., 2009. Mechanics and velocity of the Lärchberg-Galgenwald landslide (Austria). Engineering Geologydoi:10.1016/j.enggeo.2009.01.002, in press. Stead, D., Eberhardt, E., Coggan, J. S., 2006. Developments in the characterization of complex rock slope deformation and failure using numerical modelling techniques. Engineering Geology 83, 217-235. Wyllie, D. C., Mah, C. W., 2004. Rock Slope Engineering: Civil and Mining. UK Spon Press, London.

• However, a connected failure surface is only achieved once a secondary rupture has propagated downwards into this slip plane from the upper breakaway zone.

Pasculli, A., Sciarra, N., 2006. A 3D landslide analyses with constant